ISO A1 841mm x 594mm

A) GENERAL :-

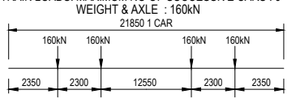
1. WHERE REFERENCE IS MADE TO ANY DRAWINGS THE LATEST REVISION OF THAT DRAWING SHALL BE USED. DRAWINGS SHALL BE READ IN CONJUNCTION WITH OTHER CONCERNED RELEVANT DRAWINGS.
2. DIMENSIONS ARE NOT TO BE SCALED. ONLY WRITTEN DIMENSIONS SHALL BE FOLLOWED.

B) LEVELS AND DIMENSIONS :-

1. ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE STATED.
2. ALL LEVELS AND CHAINAGES ARE IN METERS UNLESS OTHERWISE SPECIFIED.

C.) DESIGN LOADS

1. THE BRIDGE STRUCTURE ARE DESIGNED IN ACCORDANCE TO IRC : 6 AND IRC : 112 TO CARRY THE FOLLOWING LOADINGS:
   a. DEAD LOAD: (SELF WEIGHT AND SUPER IMPOSED DEAD LOADS)
      REINFORCED/PRESTRESSED CONCRETE = 25.0 kN/m^3
      PREMIX (MAXIMUM DESIGN THICKNESS = 100mm) = 23.0 kN/m^3
      STEEL = 78.5 kN/m^3
   b. TRAIN LOADS: MAXIMUM NO OF SUCCESSIVE CARS : 6
      WEIGHT & AXLE : 160kN



   c. SUPERIMPOSED DEAD LOAD: SIDL MAXIMUM = 8.16 t/m FOR TWO TRACK LOAD.
   d. HIGHWAY VEHICULAR LIVE LOADS:
      i. TWO LANES OF CLASS A OR ONE LANE OF 70R WHICHEVER IS GOVERNING.
   e. WIND LOADS: AS PER IS - 875 (PART 3) BASED ON A MEAN HOURLY WIND SPEED OF 39m/s.
   f. TEMPERATURE EFFECTS: MAXIMUM TEMP. = 47.8°C, MEAN TEMP. = 22.80°C
      MINIMUM TEMP. = 2.2°C
   g. SEISMIC : CONSIDERED AS PER RDSO SEISMIC CODE

D.) CONCRETE CASTING :-

1. CONCRETE IN THE PILE SHALL BE CAST TO FULL DEPTH IN SINGLE POUR.
2. CONCRETING FOR PILE CAP SHALL BE CARRIED OUT TO THE TOP OF THE PILE CAP IN SINGLE POUR. SUBSEQUENTLY STARTER OF PIER SHALL BE MADE.
3. CONCRETING IN PIER SHALL BE CAST FROM STARTER OF PIER TO TOP OF PIER CAP IN SINGLE POUR.
4. FORMWORK FOR FULL HEIGHT INCLUDING PIER CAP SHALL BE ERECTED. BOOM PLACER TOWER CRANE SHALL BE USED FOR POURING CONCRETE. TREMIE PIPES OF 300mm DIA WITH FUNNEL AT TOP WILL BE LOWERED INTO POSITION. CONCRETE SHALL BE COMPACTED BY FORM VIBRATORS AS WELL AS NEEDLE VIBRATORS. A METHOD STATEMENT SUPPORTED BY RELEVANT DRAWINGS SKETCHES OF ALL OPERATIONS PROPOSED BY CONTRACTOR REQUIRED TO FULFILL ABOVE OBJECTIVE SHALL BE SUBMITTED FOR APPROVAL OF ENGINEER IN-CHARGE.
5. NORMALLY NO HOLES SHALL BE PERMITTED FOR FIXING THE SHUTTERING ETC. HOWEVER ANY HOLES RECESS LEFT IN THE CONCRETE FOR CONSTRUCTION ACTIVITY SHALL BE FILLED WITH NON-SHRINK EXPANSIVE PREPACKED MORTAR TO THE SATISFACTION OF ENGINEER IN-CHARGE.

E.) CONCRETE :-

1. CONCRETE SHALL BE OF DESIGN MIX AND HAVE A MINIMUM 28 DAYS CHARACTERISTIC STRENGTH AS SPECIFIED BELOW OR IN RELEVANT GFC DRAWINGS:
   PRECAST U-GIRDER INCLUDING DOWNSTAND - M55
   PRECAST PIERCAP - M50
   CAST-IN-SITU SEGMENTAL BOX GIRDER INCLUDING DOWNSTAND - M55
   PARAPET - M50
   BEARING PAD (MORTAR) - M75
   BEARING UPSTAND - M50
   CAST-IN-SITU PIERCAP - M50
   PIERS / PORTALS - M50
   PILECAP / OPEN FOUNDATION - M35
   PILES - M35
2. THE NOMINAL MAXIMUM SIZE OF AGGREGATE TO BE USED IN RCC AND PSC WORK SHALL BE 20mm.
3. TO IMPROVE WORKABILITY OF CONCRETE AND CEMENT GROUT, ADMIXTURES CONFORMING TO IS:9103 AND TESTED IN ACCORDANCE WITH IS:6925 COULD BE PERMITTED SUBJECT TO SATISFACTORY PROPER USE. ADMIXTURES GENERATING HYDROGEN, NITROGEN, CHLORIDES ETC SHALL NOT TO BE USED. NO ADMIXTURE SHALL BE USED WITHOUT THE APPROVAL OF ENGINEER IN-CHARGE. NO EXTRA PAYMENT SHALL BE ADMISSIBLE ON THIS COUNT.
4. FOLLOWING CLEAR COVER SHALL BE PROVIDED FOR DIFFERENT STRUCTURAL ELEMENTS.

FOUNDATION (PILE / PILECAP / OPEN FOUNDATION)	- 75mm
PORTAL, PIER CAP, PIER & BEARING UPSTAND	- 75mm
PRECAST U-GIRDER, I-GIRDER	- 50mm
CAST-IN-SITU BOX GIRDER	- 50mm
CAST-IN-SITU DECKSLAB	- 35mm
CAST-IN-SITU DIAPHRAGM & CAST-IN-SITU STITCH TO PARAPET	- 50mm
PRECAST PARAPET	- 50mm

F.) WATER :-

1. WATER TO BE USED IN CONCRETING, GROUTING AND CURING ETC. AND SHALL CONFORM TO IS:456-2000IRC:SP:33-1989 HOWEVER, CHLORIDE CONTENT IN WATER SHALL NOT TO EXCEED 500mgLITRE.

G.) REINFORCEMENT :-

1. REINFORCING STEEL SHALL BE OF HYSD BARS (GRADE DESIGNATION FE550D) CONFORMING TO IS:1786, ONLY TMT BAR SHALL BE PROVIDED.
2. STEEL SPACER BARS SHALL BE PROVIDED BETWEEN ADJACENT LAYERS OF PARALLEL REINFORCEMENT AND SPACED AT NOT MORE THAN 60XSMALLER BAR DIA. THE DIAMETER OF THE SPACER BAR SHALL BE AT LEAST 25MM BUT NOT LESS THAN THE DIA OF THE PARALLEL REINFORCEMENTS.
3. RINGS CHAIRS OR OTHER ARRANGEMENTS WHICH MAY BE REQUIRED TO SUPPORT REINFORCEMENT AND WHICH ARE NOT REQUIRED AS PART THE PERMANENT DESIGN ARE NOT SHOWN ON THE DRAWINGS. RATE OF REINFORCEMENT INCLUDES ALL SUCH ITEMS AND NOTHING EXTRA PAYMENT SHALL BE ADMISSIBLE TO THE CONTRACTOR.
4. BENDING OF REINFORCEMENT BARS TO BE AS PER IS:1963IS:2502, WITH LIMIT OF SPANDREL DIAMETER OF 8 TIMES DIAMETER OF BAR IF NOT MENTIONED.
5. G.I WIRES SHALL BE USED FOR BINDING REINFORCEMENT.
6. REINFORCEMENT SHALL BE FABRICATED IN CAGES FOR PILES, AND THEN BROUGHT IN TO POSITION FOR CONSTRUCTION.
7. ALL REINFORCEMENT BAR JUNCTIONS SHALL BE BOUND BY TWO PLY OF BINDING WIRES IN A DIAMOND FASHION PASSING BELOW DIAGONALS. ALL REINFORCEMENT LAPPING AND COUPLING SHALL BE AS PER TECHNICAL SPECIFICATIONS.
8. LAP SPLICE SHOULD BE STAGGERED AND NO WHERE MORE THAN 50 OF REINFORCEMENT SHALL BE LAPPED AT ANY ONE LOCATION.
9. IN ADDITION TO THIS STRUCTURAL STEEL REINFORCEMENT, NON STRUCTURAL EARTHING REBARS SHALL BE PROVIDED IN REBARS SHALL BE THE SAME AS FOR STRUCTURAL REBARS.
10. THE DEVELOPMENT LENGTH SHALL BE PROVIDED FOR FE550D GRADE OF STEEL AS FOLLOWS:

DEVELOPMENT LENGTH (Ld) IN TENSION

GRADE OF CONCRETE	DIAMETER OF BAR						
	10	12	16	20	25	28	32
M35	460	550	735	915	1150	1290	1470
M40 & ABOVE	420	510	675	840	1050	1175	1350

DEVELOPMENT LENGTH (Ld) IN COMPRESSION

GRADE OF CONCRETE	DIAMETER OF BAR						
	10	12	16	20	25	28	32
M35	370	450	600	750	925	1050	1190
M40 & ABOVE	400	480	640	800	1000	1120	1250

H.) PRESTRESSING :-

1. THE SUPERSTRUCTURE DESIGN IS BASED ON THE USE OF INTERNAL POST-TENSIONING SYSTEMS. PRE-STRESSING SYSTEMS PROPOSED BY THE CONTRACTOR SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER IN-CHARGE.
2. PERMANENT PRESTRESSING CABLES CONSISTING OF 15.2mm DIA 7 PLY, CLASS-2 LOW RELAXATION STRANDS AS PER IS:14268-1955 SHALL BE USED. THE DESIGN IS BASED ON THE FOLLOWING STRAND, PROPERTIES,

   NOMINAL DIAMETER OF THE EACH STRAND=15.2mm.
   NOMINAL AREA OF STRAND=140mm^2
   ULTIMATE TENSILE STRENGTH =1860N/mm^2
   ANCHORAGE TYPE-19T15, 12T15 & 7T15.
   SHEATHING TYPE-HDPE.
   SHEATHING DIAMETER- 69mm (ID)FOR 7T15.
   85mm (ID)FOR 12T15.
   107mm (ID)FOR 19T15

   CO-EFFICIENT OF FRICTION-0.17 / rad.
   WOBBLE CO-EFFICIENT-0.002 / m.
   MODULUS OF ELASTICITY OF STRAND-1.95X10^5 MPA
   SLIP-6mm AT STRESSING END.
   MIN-ULTIMATE BREAKING LOAD OF STRAND-261.44kN.
3. THE RELAXATION LOSS FROM AN INITIAL STRESS OF 75% OF THE ULTIMATE TENSILE STRENGTH SHALL NOT EXCEED 2.5% AFTER 1000 HOURS AT 20°c.
4. MANUFACTURER'S CERTIFICATE OF THE MATERIALS TESTS (NOT OLDER THAN ONE YEAR) SHALL BE OBTAINED PRIOR TO PRESTRESSING OPERATION. THIS WILL ALSO INCLUDE SATISFACTORY RESULTS OF THE TESTS AS PER FIP:1993 RECOMMENDATIONS LOAD TRANSFER TEST AND ANCHORAGE EFFICIENCY TEST SHALL BE CONDUCTED AS DEFINED IN FIP:1993 ENGINEER IN-CHARGE SHALL SELECT AT RANDOM, THE REQUIRED ANCHORAGES WEDGES SAMPLE FROM COMPLETED LOTS FOR TESTING BY THE MANUFACTURER. THE UNIT OF REQUIRED SIZE REINF., WILL BE REQUIRED FOR THE LOAD TRANSFER TEST.
5. A METHOD STATEMENT SUPPORTED BY RELEVANT DRAWINGS SKETCHES OF ALL PRESTRESSING OPERATIONS (SUCH AS POSITIONING OF SHEATHING WHILE CASTING OF PRE CAST SEGMENT,THREADING AND TENSIONING OF PERMANENT CABLES COMPLETE PREPARATIONS AND ARRANGEMENTS FOR GROUTING, STAGE FILLING OF ANCHORAGE, RECESS ETC.) PROPOSED BY THE CONTRACTOR'S AND HIS SPECIALIST PRESTRESSING AGENCY SHALL BE SUBMITTED FOR APPROVAL OF ENGINEER IN-CHARGE.
6. PRESTRESSING SYSTEM CONTRACTOR'S AUTHORIZED TECHNICAL REPRESENTATIVE SHALL BE REQUIRED TO SUPERVISE EVERY OPERATION RELATING TO LAYING, STRESSING, WATER TESTING AND GROUTING OF PRESTRESSING CABLES.
7. FOLLOW THE NOTES IN RELEVANT PRESTRESSING DRAWINGS FOR PRESTRESSING SEQUENCE AND OPERATIONS.
8. ALL TENDONS SHALL BE STRESSED FROM ONE END UNLESS OTHERWISE STATED IN DRAWINGS AT A GRADUAL AND STEADY RATE UNTIL THE FORCE IN THE TENDON IMMEDIATELY BEHIND THE ANCHORAGE IS AS SHOWN ON APPROVED DRAWINGS.
9. ALL ANCHORAGE PLATES ARE TO BE SET AT RIGHT ANGLES TO THE TENDON AXIS AT ANCHORAGE LOCATION.
10. THE JOINTS OF ALL SHEATHING SHALL BE TESTED AS PER IRS SPECIFICATION.
11. ALL SETTING OUT DIMENSIONS, REDUCED LEVELS, CONCRETE DIMENSIONS & CABLE PROFILES TO BE VERIFIED ON SITE BEFORE CONSTRUCTION COMMENCES. ANY DISCREPANCY TO BE BROUGHT TO THE NOTICE OF ENGINEER-IN-CHARGE IMMEDIATELY.
12. PRESTRESSING STEEL SHALL BE ACCURATELY LOCATED AND MAINTAINED IN POSITION AS PER DRAWING, WITHIN A MAXIMUM TOLERANCE OF ±3.2mm IN PRECAST GIRDER AND ±5MM FOR CAST-IN-SITU GIRDER. WHEREVER NECESSARY, REINFORCEMENT BARS MAY BE BENT OR SHIFTED LOCALLY TO AVOID CLASHING WITH PRESTRESS TENDONS AND ANCHORAGES WITHIN MAXIMUM TOLERANCE AS MENTIONED ABOVE. THE SHEATHING SHALL BE SUPPORTED AND FIXED AT A INTERVAL OF NOT EXCEEDING 0.75M BY FIXING STEEL CHAIRS ON SPACERBARS TO SECONDARY REINFORCEMENT IN SUCH A MANNER THAT THE PROFILES OF THE CABLES IS IN NO WAY DISTURBED BY HEAVY VIBRATION ANDOR BY THE PRESSURE OF WET CONCRETE. ALL PRESTRESSING STEEL SHALL BE SUITABLY CONNECTED TO THE UNTENSIONED REINFORCEMENT IN ORDER TO IMPLEMENT STRAY CURRENT PROTECTION.
13. MINIMUM STRENGTH REQUIREMENT OF CONCRETE AT THE TIME OF TENSIONING OF CABLES;
    POST TENSIONING OF A PRECAST GIRDER REQUIRES MINIMUM 7 DAYS AFTER CASTING OR 80% OF SPECIFIED CHARACTERISTIC CONCRETE STRENGTH,WHICHEVER IS LATER.

14 . PRESTRESSING SHALL BE APPLIED IN STAGES AS SPECIFIED IN PRESTRESSING DRAWINGS.

15. IF THE CALCULATED ELONGATION IS REACHED BEFORE THE CALCULATED GAUGE PRESSURE IS OBTAINED, CONTINUE TENSIONING TILL THE CALCULATED GAUGE PRESSURE IS ATTAINED, PROVIDED THE ELONGATION DOES NOT EXCEED 1.05 TIMES THE CALCULATED ELONGATION. IF THIS ELONGATION IS ACHIEVED BEFORE THE CALCULATED GAUGE PRESSURE IS ATTAINED, STOP STRESSING AND INFORM THE ENGINEER. IF THE CALCULATED ELONGATION HAS NOT BEEN REACHED CONTINUE TENSIONING BY INTERVALS OF 5kg/ SQCM TILL THE CALCULATED ELONGATION IS ATTAINED, PROVIDED THE GAUGE PRESSURE DOES NOT EXCEED 1.05 TIMES THE CALCULATED GAUGE PRESSURE. IF THE ELONGATION AT 1.05 TIMES THE CALCULATED GAUGE PRESSURE IS LESS THAN 0.95 TIMES THE CALCULATED ELONGATION, THE FOLLOWING MEASURES MUST BE TAKEN, IN SUCCESSION, TO DEFINE THE CAUSE OF THIS LACK OF ELONGATION.

- RECALIBRATE THE PRESSURE GAUGE
- CHECK THE FUNCTIONING OF THE JACK, PUMP AND LEADS
- DE-TENSION THE CABLE. SLIDE IT IN ITS DUCT TO CHECK THAT IT IS NOT BLOCKED BY MORTAR WHICH HAS ENTERED THROUGH THE HOLES IN THE SHEATH. RE-TENSION THE CABLE, IF FREE.

IF THE REQUIRED ELONGATION IS NOT OBTAINED, FURTHER FINISHING OPERATIONS SUCH AS CUTTING OR SEALING SHOULD NOT BE UNDERTAKEN WITHOUT THE APPROVAL OF THE ENGINEER.

16. PRIOR TO GROUTING, ALL CABLES SHALL BE TESTED WITH WATER PRESSURE OF 0.3MPA, FOR APPROXIMATELY 3 MINUTES TO INVESTIGATE LEAKAGES AND CONNECTIVITY OF DUCTS.
17. GROUTING SHALL BE CARRIED OUT AS PER TENDER SPECIFICATION
18. EXTRA LENGTH OF STRANDS PROJECTING BEYOND END BLOCK SHALL NOT BE CUT AFTER STRESSING PRIOR TO APPROVAL OF STRESSING OPERATION. THESE SHALL BE CUT ONLY WHEN INSTRUCTIONS FOR GROUTING ARE ISSUED BY ENGINEER-IN-CHARGE. PROTECTION OF END ANCHORAGES WILL BE DONE AS GIVEN IN SPECIFICATION.
19. SIZE OF HDPE SEATHING DUCT SHALL BE AS PAR CLAUSE ON 7.2 .64.2 OF IRS-CBC.

I.) BEARINGS

1. BEARINGS SHALL BE PROPOSED AS SPECIFIED IN THE DRAWINGS. BEARING FROM MANUFACTURER APPROVED BY THE ENGINEER INCHARGE ONLY SHALL BE PERMITTED.
2. ALL BEARINGS MUST BE REPLACEABLE WITHOUT DAMAGE TO THE STRUCTURE.
3. EACH BEARING SHALL BE INDELIBLY MARKED WITH THE BEARING IDENTIFICATION MARK. DIRECTION OF MOVEMENT OF EACH BEARING SHALL BE IDENTIFICABLE ON THE TOP FACE OF BEARING. A SCALE SHALL BE FIXED / PAINTED ON THE BEARING FOR ASSESSING MOVEMENTS.

J.) STRUCTURAL STEEL

1. THE STRENGTH OF STRUCTURAL STEEL, BOLTS AND STUDS SHALL BE AS SPECIFIED IN RELEVANT GFC DRAWINGS.
2. DIMENSIONS SHOWN IN GFC DRAWINGS DO NOT TAKE INTO ACCOUNT OF CAMBER, FABRICATION TOLERANCE EFFECTS AND WELL SHRINKAGE.

K.) ERECTION, HANDLING OF PRECAST UNITS

1. FULL SPAN PRECAST UNITS SHOULD BE MAINTAINED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE STORED, LIFTED AND / OR MOVED IN A MANNER TO PREVENT TORSION AND OTHER UNDUE STRESSES AND STRAIN.
2. THE STRESSES INDUCED IN UNITS DURING ANY STAGES OF CONSTRUCTION SHALL NOT EXCEED 50% OF THE CUBE STRENGTH ACHIEVED AT THAT STAGE. THE UNITS DURING STACKING/CURRING, LIFTING SHALL ALWAYS BE SUPPORTED AT LOCATION SHOWN IN DRAWINGS.

L.) TOLERANCES FOR CONSTRUCTION

1. TOLERANCES FOR BORED PILES
   a.) VARIATION IN CROSS-SECTIONAL DIMENSIONS : +50 mm, -10 mm
   b.) VARIATION FROM VERTICAL OR SPECIFIED RAKE VERTICAL PILE : 1.5% FOR
   c.) VARIATION IN THE FINAL POSITION OF THE HEAD IN PLAN : 75 mm
   d.) VARIATION OF LEVEL OF TOP OF PILES : ± 25 mm
2. TOLERANCES FOR PILE CAPS
   a.) VARIATION IN DIMENSIONS : +50 mm, -10 mm
   b.) MISPLACEMENT FROM SPECIFIED POSITION IN PLAN : 15 mm
   c.) SURFACE IRREGULARITIES MEASURED WITH 3 M STRAIGHT EDGE : 5 mm
   d.) VARIATION OF LEVEL AT THE TOP : ± 25 mm
3. TOLERANCES FOR OTHER CONCRETE ELEMENTS (PIERS, BEARING PLINTH, ETC)
   a.) VARIATION IN CROSS-SECTIONAL DIMENSIONS : +10 mm, -5 mm
   b.) MISPLACEMENT FROM SPECIFIED POSITION IN PLAN : 10 mm
   c.) VARIATION OF LEVEL AT THE TOP : ± 10 mm
   d.) VARIATIONS OF REDUCED LEVELS OF BEARING AREAS : ± 5 mm
   e.) VARIATIONS FROM PLUMB OVER FULL HEIGHT : ± 10 mm
   f.) SURFACE IRREGULARITIES MEASURED WITH 3 M STRAIGHT EDGE
   ALL SURFACES EXCEPT BEARING AREAS : 5 mm
   BEARING AREAS : 3 mm

M.) SHOP DRAWINGS FOR PRECAST ELEMENTS U-GIRDER, I-GIRDERS & PIERCAPS

1. THE CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS FOR APPROVAL OF ENGINEER INCHARGE. THE SHOP DRAWINGS SHALL BE BASED ON EXECUTION DRAWINGS ISSUED TO THE CONTRACTOR AND SHALL INCLUDE.

a.) FULLY AND ACCURATELY DIMENSIONED VIEWS SHOWING THE GEOMETRY OF UNITS INCLUDING ALL PROJECTIONS, RECESSES, NOTCHES, OPENINGS, BLOCKOUTS, ANY ITEMS TO BE EMBEDDED IN THE UNITS SUCH AS INSERTS, LIFTING DEVICES SHALL BE SHOWN.

b.) GEOMETRY OF UNITS SHALL TAKE ACCOUNT OF VERTICAL CURVES OR HORIZONTAL CURVES OF ALIGNMENT AS ISSUED BY CONSULTANT.

N.) AS BUILT SURVEY

1. FOLLOWING INTERIM SURVEY SHALL BE CARRIED OUT DURING CONSTRUCTION OF ELEVATED STRUCTURE

SURVEY - 1 - AFTER CONSTRUCTION OF PIER CAP AND PEDESTAL TO DETERMINE EXACT SPAN AND LEVELS.
SURVEY - 2 - AFTER COMPLETION OF DECK ERECTION TO DETERMINE EXACT CONFIGURATION FOR SECONDARY POUR CONCRETE OVER DECK SLAB.

O.) QUALITY ASSURANCE MANUAL/METHOD STATEMENT

1. CONTRACTOR SHALL SUBMIT A QUALITY ASSURANCE MANUAL/METHOD STATEMENT OF ALL OPERATIONS REQUIRED TO COMPLETE THE JOB.

REFERENCE DRAWINGS / NOTES:-

REVISIONS					
REV.	DATE	BY	DESCRIPTION	CHKD	APP.

BY DETAIL DESIGN CONSULTANT

DESIGNED BY	CHECKED BY	APPROVED BY	DATE

DETAIL DESIGN CONSULTANT

PROJECT TITLE

CLIENT

GUJARAT METRO RAIL CORPORATION LIMITED

BLOCK NO. 1, FIRST FLOOR, KARMAYOGI BHAVAN, SECTOR 10/A, GANDHINAGAR – 382010, GUJARAT, INDIABLOCK NO. 1, FIRST FLOOR, KARMAYOGI BHAVAN, SECTOR 10/A, GANDHINAGAR – 382010, GUJARAT, INDIA

DRAWING TITLE

DWG ISSUE RECORD	DWG STATUS
	✓

DRAWING NUMBER

REV. | SCALE

CAD File Name :

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